

AD-A142 736

NATIONAL PROGRAM FOR INSPECTION OF NON-FEDERAL DAMS  
PRITCHARDS POND DAM (..(U) CORPS OF ENGINEERS WALTHAM  
MA NEW ENGLAND DIV JAN 81

1/1

UNCLASSIFIED

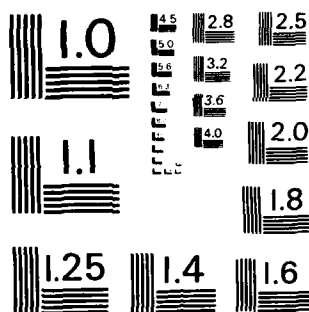
F/G 13/13

NL

END

DATE  
FILMED  
8 '84

DTIC 5



MICROCOPY RESOLUTION TEST CHART  
NATIONAL BUREAU OF STANDARDS-1963-A

AD-A142 736

PRITCHARDS POND DAM  
CT 00033

NAUGATUCK RIVER BASIN  
WATERBURY, CONNECTICUT

DTIC  
ELECTRONIC  
S JUL 2 1984  
A

This document has been approved  
for public release and its  
distribution is unlimited

PHASE I INSPECTION REPORT  
NATIONAL DAM INSPECTION REPORT

84 06 29 074

DTIC FILE COPY

**UNCLASSIFIED**

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1. REPORT NUMBER CT 00033	2. GOVT ACCESSION NO. A2-A142-736	3. RECIPIENT'S CATALOG NUMBER
4. TITLE (and Subtitle) Naugatuck River Basin Waterbury, Conn. NATIONAL PROGRAM FOR INSPECTION OF NON-FEDERAL DAMS		5. TYPE OF REPORT & PERIOD COVERED INSPECTION REPORT
7. AUTHOR(s) U.S. ARMY CORPS OF ENGINEERS NEW ENGLAND DIVISION		6. PERFORMING ORG. REPORT NUMBER
9. PERFORMING ORGANIZATION NAME AND ADDRESS		8. CONTRACT OR GRANT NUMBER(s)
11. CONTROLLING OFFICE NAME AND ADDRESS DEPT. OF THE ARMY, CORPS OF ENGINEERS NEW ENGLAND DIVISION, NEDED 424 TRAPELO ROAD, WALTHAM, MA. 02254		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS
14. MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office)		12. REPORT DATE January 1981
		13. NUMBER OF PAGES 34
		15. SECURITY CLASS. (of this report)  UNCLASSIFIED
		15a. DECLASSIFICATION/DOWNGRADING SCHEDULE
16. DISTRIBUTION STATEMENT (of this Report)  APPROVAL FOR PUBLIC RELEASE: DISTRIBUTION UNLIMITED		
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report)		
18. SUPPLEMENTARY NOTES Cover program reads: Phase I Inspection Report, National Dam Inspection Program; however, the official title of the program is: National Program for Inspection of Non-Federal Dams; use cover date for date of report.		
19. KEY WORDS (Continue on reverse side if necessary and identify by block number) DAMS, INSPECTION, DAM SAFETY, Pritchards Pond Dam Naugatuck River Basin Waterbury, Conn.		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) Pritchards Pond Dam is an embankment dam formed by Pearl Lake Road. It has a total length of 249 ft. and a maximum height of 8.7 ft. The exact age of the dam is not known but it is believed to be at least 100 years old. There is a no longer functioning outlet box located on the right side of the dam that presumably controlled a 6-inch cast iron outlet pipe on the downstream side of the dam. There is a bar screen and 4 ft. wide c. overflow spillway located in the center of the dam. This spillway drops down to a 15 inch pipe which outlets at the downstream side of the dam. The downstream side has a stone masonry wall along approx. 90 ft. of the dam's length, with varying heights.		

*Philip W. Genovese and Associates, Inc.*  
*Consulting and Design Engineers*

January 6, 1981

Re: Pritchards Pond Dam  
Waterbury, Connecticut  
Contract #DACW-33-81-C0017

The Department of the Army  
New England Division  
Corps of Engineers  
424 Trapelo Road  
Waltham, Massachusetts 02154

Attention: Mr. E. P. Gould, Project Management Division

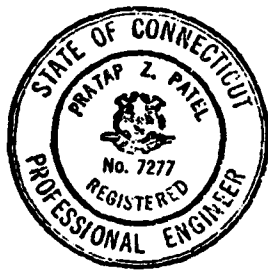
Gentlemen:

We have inspected Pritchards Pond Dam and conducted a field survey. Our dam failure analysis concludes that the dam should be reclassified as having a low hazard potential.

We are including with this letter a short report substantiating our conclusions.

Very truly yours,

PHILIP W. GENOVESE & ASSOCIATES, INC.



*Pratap Z. Patel*  
Pratap Z. Patel, P.E.  
Project Manager

PZP/LH



295 Treadwell Street, Hamden, Conn. 06514 P. O. Box 4330

Telephone 288-5878 (203) Cable GENOPHIL

## TABLE OF CONTENTS

	<u>PAGE</u>
Description	1
Location Map	2
Overview Photo	3
Hydrologic/Hydraulic Evaluation	4
Appendixes	
A Site Plan	A-1
B Site Photographs	B-1
C Inventory Form	C-1
D Hydrologic/Hydraulic Calculations	D-1
E Visual Check with Comments	E-1

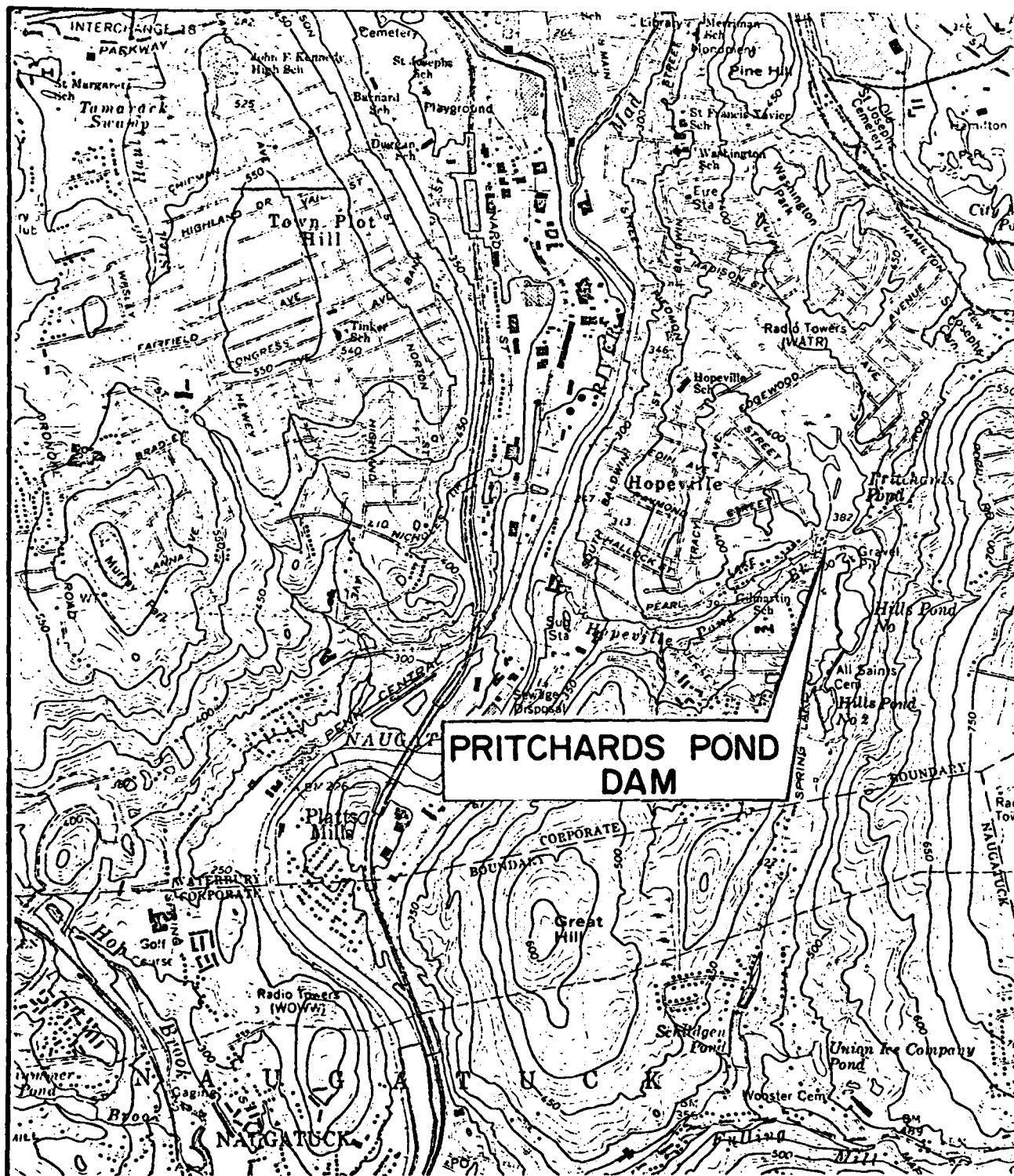
### DESCRIPTION

Name of Dam : Pritchards Pond Dam  
Identification Number : CT 00033  
Town : Waterbury  
County and State : New Haven County, Connecticut  
Stream : Hopeville Pond Brook  
Owner : Risdon Manufacturing Company, 2100 South Main Street,  
Waterbury, Connecticut  
Date of Inspection : December 3, 1980

Pritchards Pond Dam is an embankment dam formed by Pearl Lake Road. It has a total length of 249 feet and a maximum height of 8.7 feet. The exact age of the dam is not known but it is believed to be at least 100 years old. There is a no longer functioning outlet box located on the right side of the dam that presumably controlled a 6-inch cast iron outlet pipe on the downstream side of the dam. There is a bar screen and 4 foot wide overflow spillway located in the center of the dam. This spillway drops down to a 15-inch pipe which outlets at the downstream side of the dam. The downstream side has a stone masonry wall along approximately 90 feet of the dam's length, with varying heights.

The dam is owned and operated by the Risdon Manufacturing Company, 2100 South Main Street, Waterbury, Connecticut. Although it once augmented the plant's water supply, it no longer is used for that purpose. Any present uses are strictly recreational.

The dam appears in good shape but requires some work. Specifically, this would include developing a functioning outlet works, spillway maintenance and removal of trees on or close to the dam.



USGS QUAD  
WATERBURY, CT.



PHILIP W. GENOVESE AND  
ASSOCIATES, INC.  
ENGINEERS - HAMDEN, CT.

U.S. ARMY ENGINEER DIV.  
NEW ENGLAND  
CORPS OF ENGINEERS  
WALTHAM, MASS.

SCALE IN FEET  
0 1000 2000 3000 4000

NATIONAL PROGRAM OF INSPECTION OF  
NON - FED DAMS  
LOCATION MAP





U.S. ARMY ENGINEER DIV.  
NEW ENGLAND  
CORPS OF ENGINEERS  
WALTHAM, MASS.

PHILIP W. GENOVESE AND  
ASSOCIATES, INC.  
ENGINEERS, AMDEN, CT.

NATIONAL  
PROGRAM  
OF  
INSPECTION  
OF  
NON-FED  
DAMS

OVERVIEW PHOTO

DECEMBER, 1980

PRITCHARDS POND DAM

HOPEVILLE POND BROOK

WATERBURY,

CONNECTICUT

## HYDROLOGIC/HYDRAULIC EVALUATION

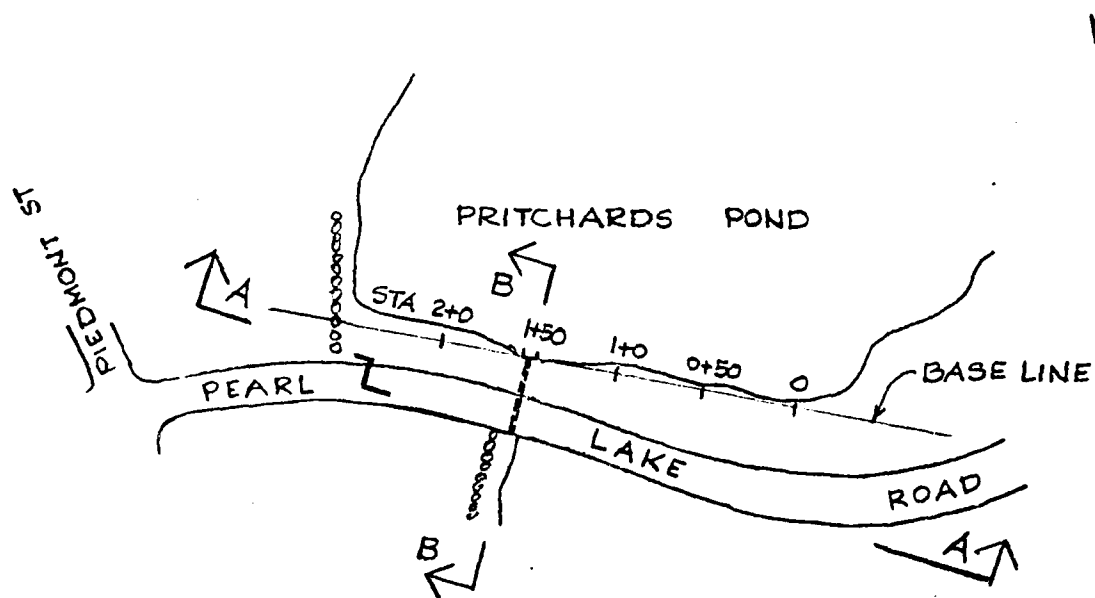
Pritchards Pond Dam has a tributary watershed of 0.25 square miles. At the spillway crest it has a water surface area of 11 acres and a storage capacity of 14 acre-feet. The storage capacity at the top of the dam is 115 acre-feet.

The pipe spillway has a capacity of 16 cfs with the water at the top of the dam. The maximum height of the dam is 8.7 feet. In accordance with the Corps of Engineers' Recommended Guidelines for Safety Inspection of Dams, Pritchards Pond Dam is a small dam based on storage capacity.

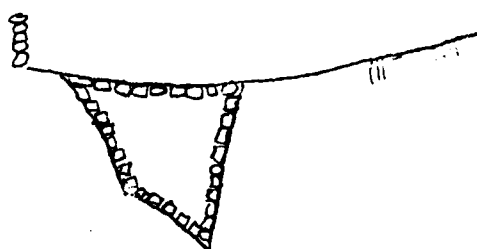
A dam breach analysis was made using the Corps of Engineers' "Rule of Thumb" guidance for estimating downstream dam failure hydrographs. The peak discharge from a dam breach, with the water level at the top of dam (elev. 386.7), was calculated to be 1200 cfs. The flood waters were routed for a distance of 3270 feet downstream.

The results of this analysis indicated that the loss of life from a failure of Pritchards Pond Dam is unlikely and therefore warrants a "low" hazard classification. Appendix D provides the detailed analysis to justify this conclusion.

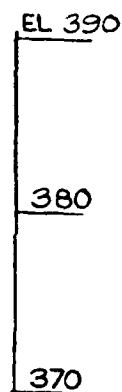
APPENDIX A  
SITE PLAN



PLAN



SECTION AA



SCALE IN FEET

PHILIP W. GENOVESE & ASSOCIATES, INC.  
ENGINEERS HAMDEN, CONNECTICUT

PRITCHARDS POND DAM (CT00000)

A-1

EL 390

380

370

PEARL LAKE ROAD

15" RCP

SECTION BB (STA 1+50)

VER 0 5' 10'  
HOR 0 10' 20'

SCALE IN FEET

PHILIP W. GENOVESE & ASSOCIATES, INC.

ENGINEERS

HAMDEN, CONNECTICUT

PRITCHARDS

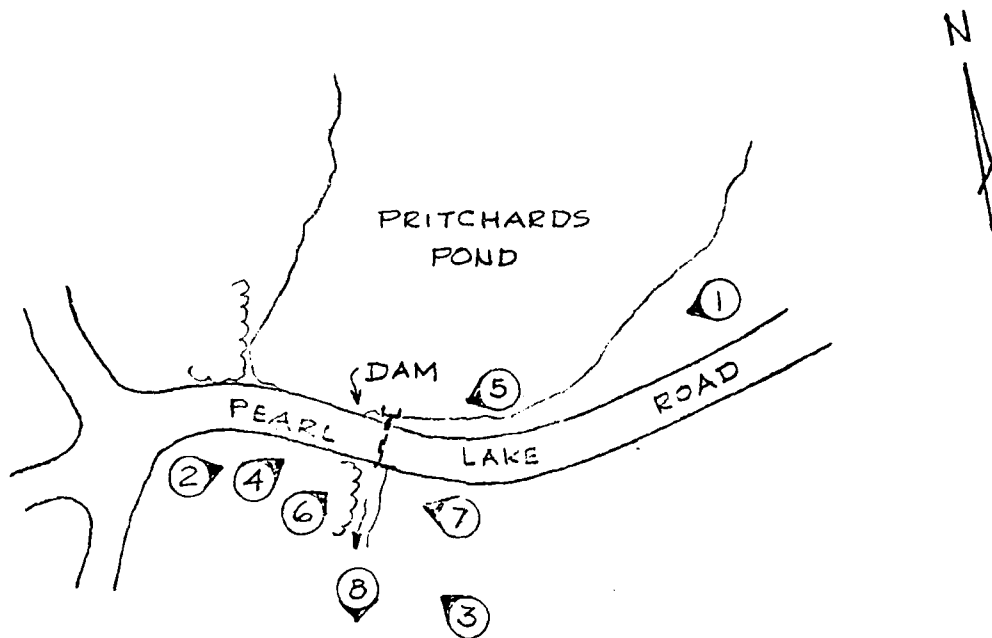
POND

DAM

(CT00033)

A-2

APPENDIX B  
SITE PHOTOGRAPHS



REFERS TO PHOTO NUMBER,  
LOCATION AND DIRECTION

U.S. ARMY ENGINEER DIV.  
NEW ENGLAND  
CORPS OF ENGINEERS  
WALTHAM, MASS.

PHILIP W. GENOVESE AND  
ASSOCIATES, INC.  
ENGINEERS - HAMDEN, CT.

NATIONAL  
PROGRAM  
OF  
INSPECTION  
OF  
NON-FED  
DAMS

## PHOTO LOCATION PLAN

PRITCHARDS POND DAM

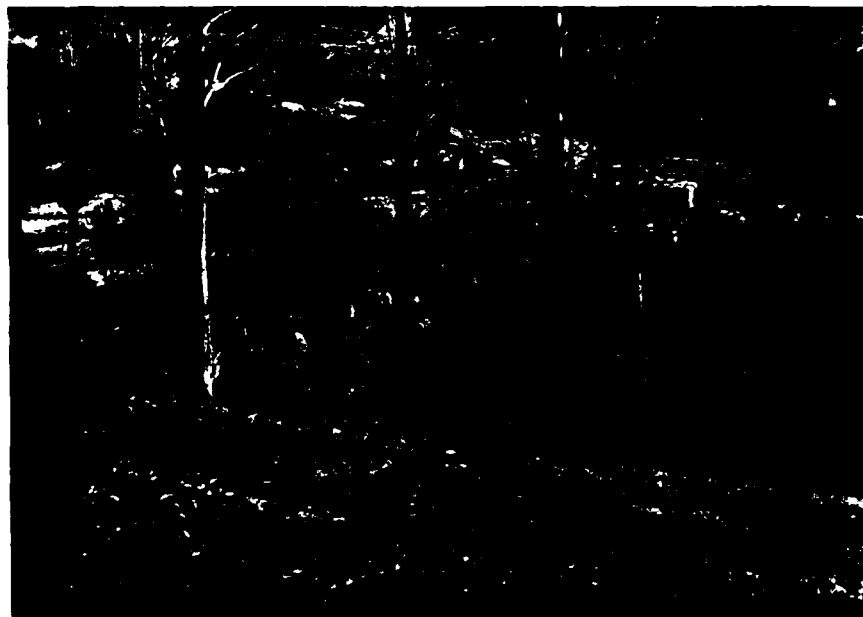
HOPEVILLE POND BROOK

WATERBURY,

CONNECTICUT



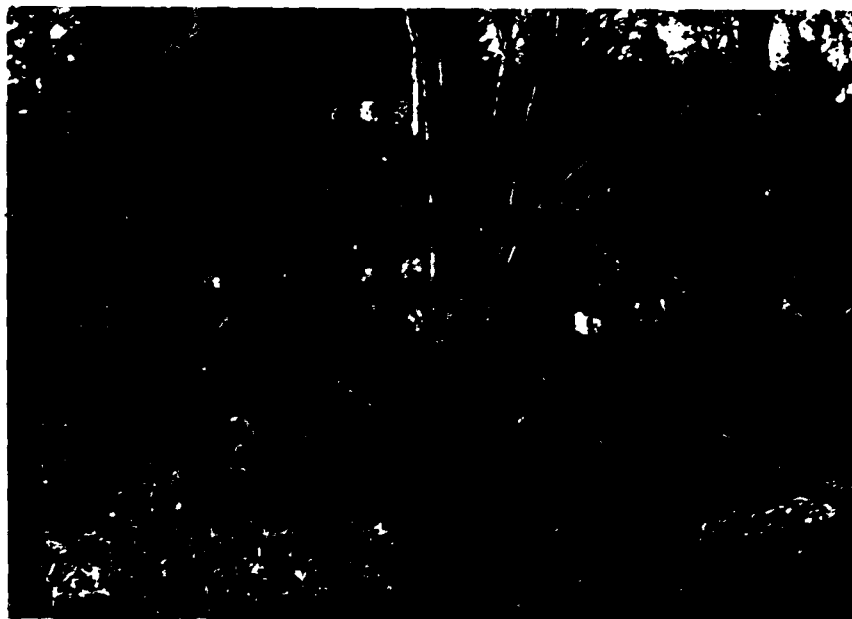
1. Left abutment, looking along crest.



2. Right abutment, looking along downstream face.

B-1





3. Downstream face of dam, looking towards right side of spillway channel. Note 14" diameter tree in right side of photo and clump of 5 trees in center of photo.



4. Sta 2+10 looking at downstream face of dam, blue flagging at Sta 2+00, tree stump on left, 8" diameter, tree on right of photo 11" diameter.

B-3



5. Spillway intake structure with trash rack.



6. Spillway and outlet discharge pipes.



7. Downstream face of dam looking towards right downstream bank.



8. Downstream channel.

B-5

PHILIP W. GENOVESE & ASSOCIATES, INC.  
ENGINEERS                      HAMDEN, CONNECTICUT

PRITCHARDS    POND    DAM    (CT00033)

APPENDIX C  
INVENTORY FORM

30 MAY 80

PAGE

AT

DA IN THE UNITED STATES

CT

IDENTITY NUMBER	33	CT	009	05	CONC	STATE	COUNTY	CITY	NAME	PRITCHARDS POND DAM	LATITUDE NORTH	73 1.6	LONGITUDE WEST	73 01.6	REPORT DATE DAY	14	MO	JUN	YR	80
-----------------	----	----	-----	----	------	-------	--------	------	------	---------------------	----------------	--------	----------------	---------	-----------------	----	----	-----	----	----

POPULAR NAME	PRITCHARDS POND						
NAME OF IMPONDMENT	PRITCHARDS POND						
REGION	01	07	TR NAUGATUCK RIVER	NEAREST DOWNSTREAM CITY-TOWN-VILLAGE	NAUGATUCK	POPULATION	236000

TYPE OF DAM	HECB	YEAR COMPLETED	1890	PURPOSES	8.7	HYDRO. HEIGHT	3.8	IMPONDING CAPACITIES	115	NEED
-------------	------	----------------	------	----------	-----	---------------	-----	----------------------	-----	------

REMARKS	
---------	--

D/S HAS	4	SPILLWAY	3	VOLUME OF DAM (CY)	0	POWER CAPACITY	0	NAVIGATION LOCKS	
---------	---	----------	---	--------------------	---	----------------	---	------------------	--

OWNER	RISON MFG CO	ENGINEERING BY		CONSTRUCTION BY	
-------	--------------	----------------	--	-----------------	--

DESIGN		CONSTRUCTION		OPERATION		MAINTENANCE	
--------	--	--------------	--	-----------	--	-------------	--

INSPECTION BY	PRITCHARDS ASSOCIATES INC.	INSPECTION DATE DAY	3	MO	DEC	YR	80	AUTHORITY FOR INSPECTION	PA 571 SECT 25-11 ST OF CT
---------------	----------------------------	---------------------	---	----	-----	----	----	--------------------------	----------------------------

REMARKS	
---------	--

APPENDIX D  
HYDROLOGIC/HYDRAULIC CALCULATIONS

## EVALUATION OF HYDRAULIC/HYDROLOGIC FEATURES

The Pritchards Pond Dam has a tributary watershed of 0.25 sq.mi and a water surface area and storage capacity at spillway level of 11 Acres and 14 Ac.Ft respectively. The maximum impoundment to the top of dam (El. 386.7 NGVD) is estimated to be 115 Ac.Ft.

The pipe spillway with drop inlet has an estimated capacity of 16 CFS with pool at top of the dam. In accordance with Table 1 of the Corps of Engineers Recommended Guidelines for Safety Inspection of Dams, the Pritchards Pond Dam is classified as "Small" in size based on storage capacity.

Utilizing the Corps of Engineers April 1978 "Rule of Thumb Guidance for Estimating Downstream Failure Hydrographs", the peak failure outflow due to dam breach is estimated to be 1200 cfs with an estimated flood depth of 3.8 Ft. immediately downstream of the dam. The flood routing was performed for peak failure outflow with pool at top of dam.

The estimated peak flow rates and peak flood depths at four sections downstream of the dam resulting from a dam failure are:

<u>D/S Section</u> (Ft. from Dam)	<u>Flow</u> (CFS)	<u>Flood Depth</u> (FT)	<u>Velocity</u> (fps)
At Dam	1200	3.8	-
170	1185	3.4	3.5
720	1148	6.2	4.1
2320	1032	4.1	4.25
3270	1021	3.2	3.9

Based on relative elevations of the houses in the vicinity of the Brook, none of them are likely to be flooded during dam failure except one house on Spring Lake Rd, located 3'4" above Brook bed which may have minor flooding. In addition, the culvert on Spring Lake Rd is inadequate to pass the peak flow of 1185 cfs.

Thus, loss of life from a failure of Pritchards Pond Dam is considered unlikely. Therefore, the dam is classified as "Low" hazard potential. This conclusion is based upon hydraulic/hydrologic analysis included in Appendix D.





# DIVERSIFIED TECHNOLOGIES CORP.

CONSULTING ENGINEERS  
NORTH HAVEN, CONN.

PROJECT NON FEDERAL DAM INSPECTION PROJECT NO. 80-13-11 SHEET 1 OF 16  
NEW ENGLAND DIVISION COMPUTED BY MA DATE 12/16/80  
PRITCHARDS POND DAM CHECKED BY ES DATE 12/17/80

FOR THE PURPOSE OF DETERMINING PROJECT SIZE, THE  
 MAXIMUM STORAGE ELEV<sup>n</sup> IS CONSIDERED AT TOP OF  
 THE DAM.

TOP OF DAM = EL. 386.7 NGVD \*  
 TOE OF DAM = EL. 378 (15" RCP OUTLET INVERT)  
HEIGHT OF DAM = 8.7 FT. (4%)

PLANIMETERING FROM USGS MAP FOR POND SURFACE AREAS —  
 AT EL. 382 (NORMAL) = 12 Ac  
 AT EL. 390 = 42 Ac

FROM STAGE-POND AREA CURVE:

POND AREA AT SPILLWAY CREST (EL. 381.8) = 11 Ac

POND AREA AT TOP OF DAM (EL. 386.7) = 30 Ac

AVERAGE POND AREA BETWEEN SPILLWAY CREST &  
 TOP OF DAM = 20.5 Ac

STORAGE BETWEEN SP. CREST & TOP OF DAM =  $4.9 \times 20.5$   
 = 101 AC·FT

ESTIMATED STORAGE BELOW SP. CREST =  $\frac{1}{3} 6.6$   
 $\frac{1}{3} \times 11 (381.8 - 378) = 14 \text{ AC·FT.}$

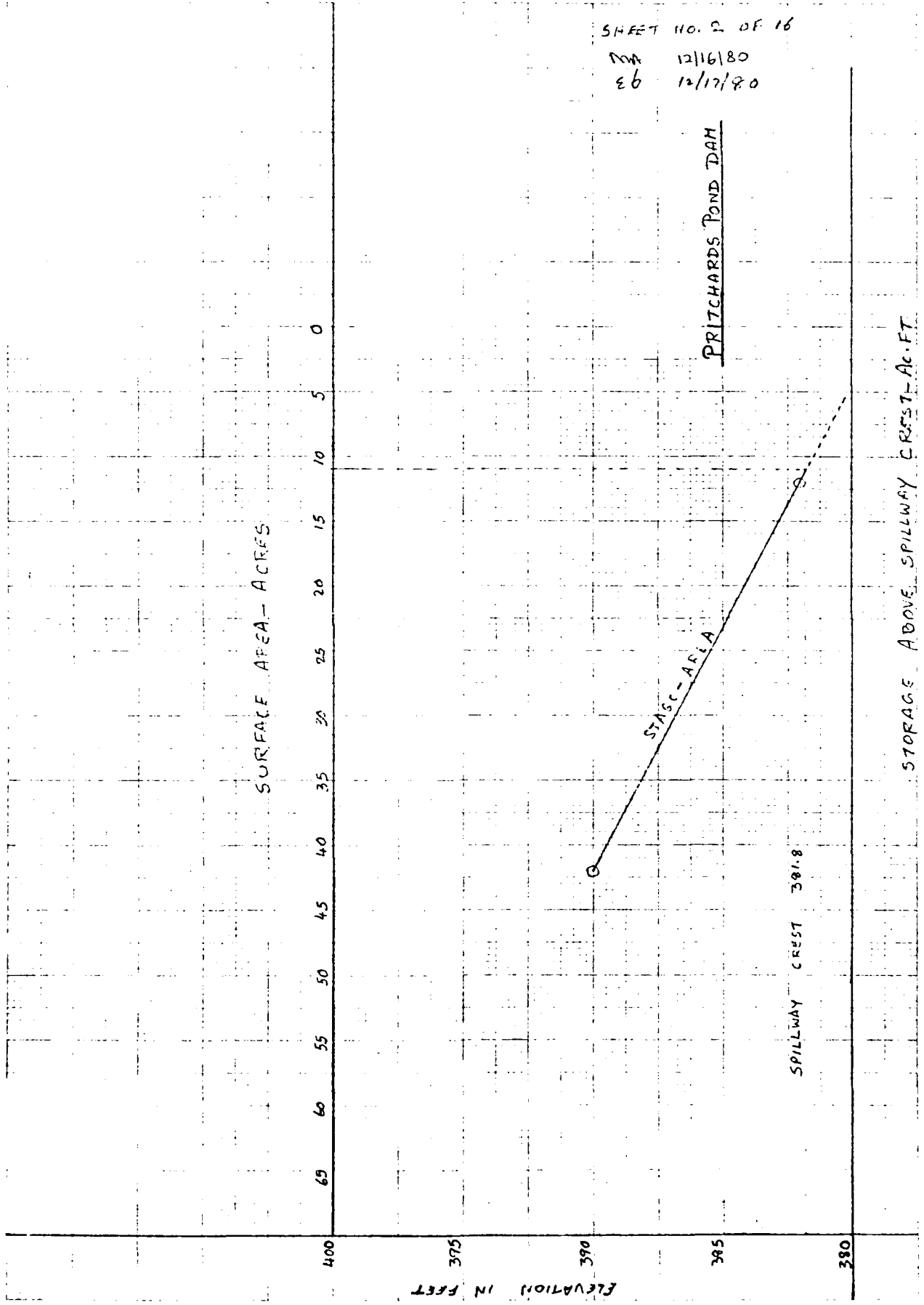
∴ MAX<sup>m</sup> IMPOUNDMENT TO TOP OF DAM = 101 + 14  
 = 115 AC·FT. (S)

\* THE WATER SURFACE ELEV<sup>n</sup> OF 382 MSL FOR PRITCHARDS  
 POND ON THE WATERBURY QUAD SHEET (1972) IS ASSUMED  
 TO BE ON NATIONAL GEODETIC VERTICAL DATUM (NGVD).  
 ALL OTHER ELEVATIONS ARE REFERENCED TO THIS  
 ASSUMED ELEV<sup>n</sup> AND ARE OBTAINED BASED UPON  
 INFORMATION FURNISHED BY P.W. GENOVESE & ASS. INC.

D-1

SHEET NO. 2 OF 16

DA 12/16/80  
EB 12/17/80



# DIVERSIFIED TECHNOLOGIES CORP.

CONSULTING ENGINEERS  
NORTH HAVEN, CONN.

PROJECT NON FEDERAL DAM INSPECTION PROJECT NO. 80-13-11 SHEET 3 OF 16  
NEW ENGLAND DIVISION COMPUTED BY MM DATE 12/16/80  
PRITCHARDS POND DAM CHECKED BY ES DATE 12/17/80

BREACH ANALYSIS - DOWNSTREAM FAILURE HAZARD  
 BASED UPON CORPS OF ENGINEERS "RULE OF  
 THUMB" GUIDANCE FOR ESTIMATING D/S DAM  
 FAILURE HYDROGRAPHS

$$\text{BREACH OUTFLOW } Q_b = \frac{8}{27} \times W_b \times \sqrt{g} \times Y_0^{3/2}$$

WATER DEPTH AT TIME OF FAILURE  $Y_0 = 8.7$  FT WITH  
 POOL AT TOP OF DAM

ESTIMATED BREACH WIDTH  $W_b = 40\%$  OF MID-HT LENGTH  
 OF DAM  
 $= 0.4 \times 67'$

(MID-HT LENGTH IS BASED UPON P.W. GENOVESE &  
 ASSOC. INC.'S DEC. 9, 1980 FIELD INFORMATION)

$$\therefore Q_b = \frac{8}{27} \times (0.4 \times 67) \times \sqrt{32.2} \times (8.7)^{3/2}$$

$$\approx 1200 \text{ CFS}$$

IT IS PRESUMED THAT THE BREACH OCCURS IN  
 DEEPEST SECTION OF THE DAM. THIS SECTION  
 INCLUDES THE PIPE SPILLWAY WITH DROP INLET.

$$\therefore \text{PEAK FAILURE OUTFLOW } Q_{P_1} = 1200 \text{ CFS}$$

ESTIMATED FAILURE FLOOD DEPTH  $\approx 0.44 Y_0$   
IMMEDIATELY D/S FROM DAM  $\approx 3.8 \text{ FT.}$

# DIVERSIFIED TECHNOLOGIES CORP.

CONSULTING ENGINEERS  
NORTH HAVEN, CONN.

PROJECT NON FEDERAL DAM INSPECTION PROJECT NO. 80-13-11 SHEET 4 OF 16  
NEW ENGLAND DIVISION COMPUTED BY DMP DATE 12/11/82  
PRITCHARDS POND DAM CHECKED BY SA DATE 12/17/82

PERFORM DIS ROUTING OF PEAK FAILURE OUTFLOW  
SECTION AA IS SELECTED 170' DIS OF THE DAM TO  
EVALUATE THE FLOOD HAZARD TO THE TWO HOUSES  
LOCATED IN BETWEEN THE BROOK AND SPRING LAKE TRS.  
USING MANNING'S EQUATION.

$$Q = \frac{1.486}{n} A R^{2/3} A^{1/2} \quad \text{WHERE } n = 0.06 \text{ ASSUMED}$$

$$= 2.724 A R^{2/3} \quad \text{AND } A = 0.012 \text{ EST. FROM USGS MAP.}$$

A AND R ARE ESTIMATED BASED ON USGS MAP INFORMATION.

ELVN	A SQ. FT.	P	R	$R^{2/3}$	Q CFS
376	0	—	—	—	—
378	105	105.1	1.0	1.0	286
379	230	154.1	1.49	1.30	817
380	415	205.2	2.02	1.60	1808

FROM STAGE-AREA AND STAGE-DISCHARGE CURVES, FOR  
SECTION AA, FOR  $Q P_1 = 1200 \text{ CFS}$ , ELVN = 379.45 AND  
AREA = 348 SQ. FT.

$$\text{VOLUME OF REACH } V_1 = \frac{170 \times 348}{43.56} \approx 1.4 \text{ AC. FT.}$$

$$\text{TRIAL } Q P_2 = Q P_1 \left(1 - \frac{V_1}{S}\right) \quad \text{WHERE } S = \text{STORAGE TO TOP OF DAM}$$

$$= 1200 \left(1 - \frac{1.4}{115}\right) = 1185 \text{ CFS}$$

FOR THIS  $Q P_2$  THE STAGE-DISCHARGE CURVE GIVES ELVN  
= 379.4 AND AREA = 342 SQ. FT.

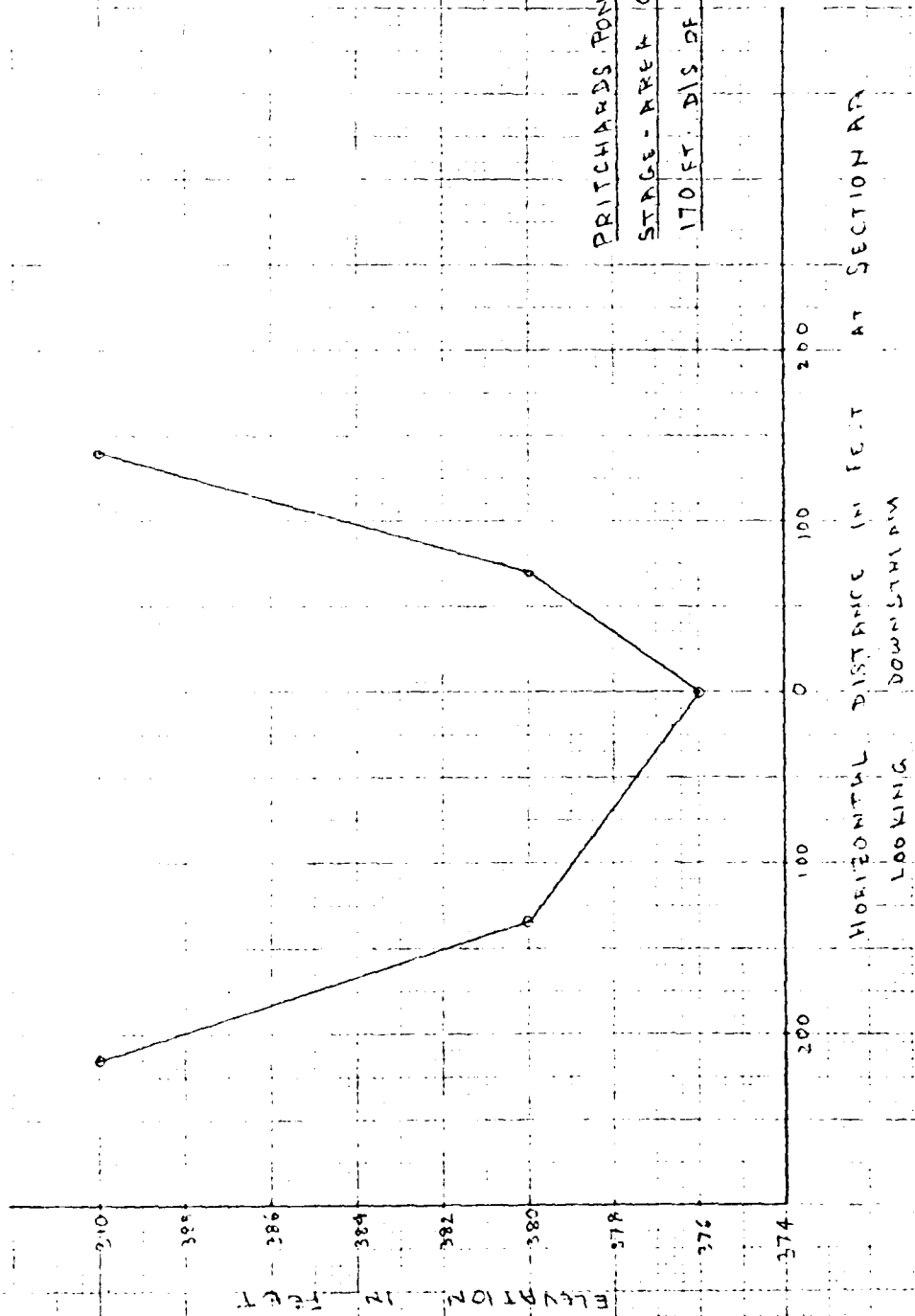
$$\text{VOLUME OF REACH } V_2 = \frac{170 \times 342}{43.56} \approx 1.4 \text{ AC. FT.}$$

$\therefore$  PEAK OUTFLOW  $Q P_2 = 1185 \text{ CFS}$   
FLOOD DEPTH AT SECTION AA =  $379.4 - 376 = 3.4 \text{ FT}$   
FLOOD STAGE AT SECTION AA = 379.4 NGVD  
AND VELDITY AT SECTION AA =  $\frac{1185}{342} = 3.5 \text{ FPS}$

SHEET NO 5 OF 16

MA 12/16/80  
EB 12/17/80

PRITCHARDS POND DAM  
STAGE - AREA CURVE  
170 FT. DIS OF DAM



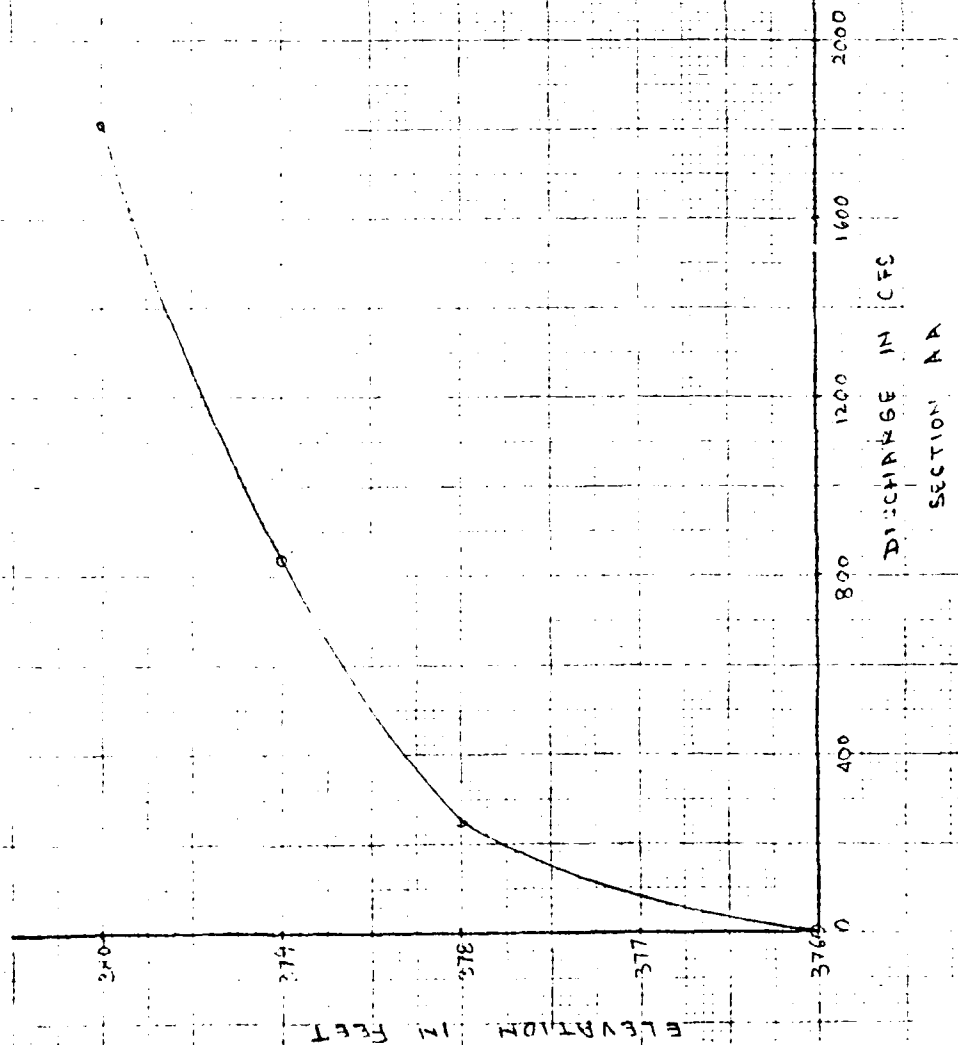
D-5

SHEET NO. 3 of 16

MA 12/16/80

EB 12/17/80

PRITCHARDS POND DAM  
STAGE - DISCHARGE CURVE



D-6

DIVERSIFIED TECHNOLOGIES CORP.

CONSULTING ENGINEERS  
NORTH HAVEN, CONN.

PROJECT NON FEDERAL DAM INSPECTION PROJECT NO. 80-13-11 SHEET 7 OF 16  
NEW ENGLAND DIVISION COMPUTED BY MA DATE 12/16/85  
PRITCHARDS POND DAM CHECKED BY CE DATE 12/17/85

OF THE TWO HOUSES, THE HOUSE AT LOWER ELEVATION IS ESTIMATED TO BE 3'4" ABOVE THE BED OF THE BROOK.

THUS, AT SECTION AA, NO SERIOUS FLOOD HAZARD IS LIKELY TO OCCUR.

IT IS HOWEVER, NOTED THAT THE CULVERT ON SPRING LAKE RD. IS INADEQUATE TO ACCOMMODATE THE ENTIRE PEAK OUTFLOW AT DAM FAILURE.

SECTION BB

THIS SECTION IS 550' BELOW SECTION AA.  
 USING MANNING'S EQUATION

$$Q = \frac{1.486}{n} A R^{2/3} V^{1/2} \quad \text{WHERE } n = 0.08 \text{ ASSUMED (SLOW MOVING?)}$$

$$= 1.948 A R^{2/3} \quad A = 0.011 \text{ EST. FROM USGS MAP}$$

ELVN	A SQ. FT	P	R	R <sup>2/3</sup>	Q CFS
370	0	—	—	—	—
372	29	29.3	0.99	0.99	56
374	116	58.6	1.98	1.58	356
376	261	87.9	2.97	2.07	1051
378	464	117.1	3.96	2.50	2263

FROM STAGE-AREA AND STAGE-DISCHARGE CURVES.  
 FOR  $Q_1 = 1185$  CFS, ELVN = 376.3 AND AREA = 290 SQ. FT  
 VOLUME OF REACH  $V_1 = \frac{550 \times 290}{43,560} \approx 3.7$  AC-FT.

$$\text{TRIAL } Q_2 = Q_1 \left(1 - \frac{V_1}{S}\right)$$

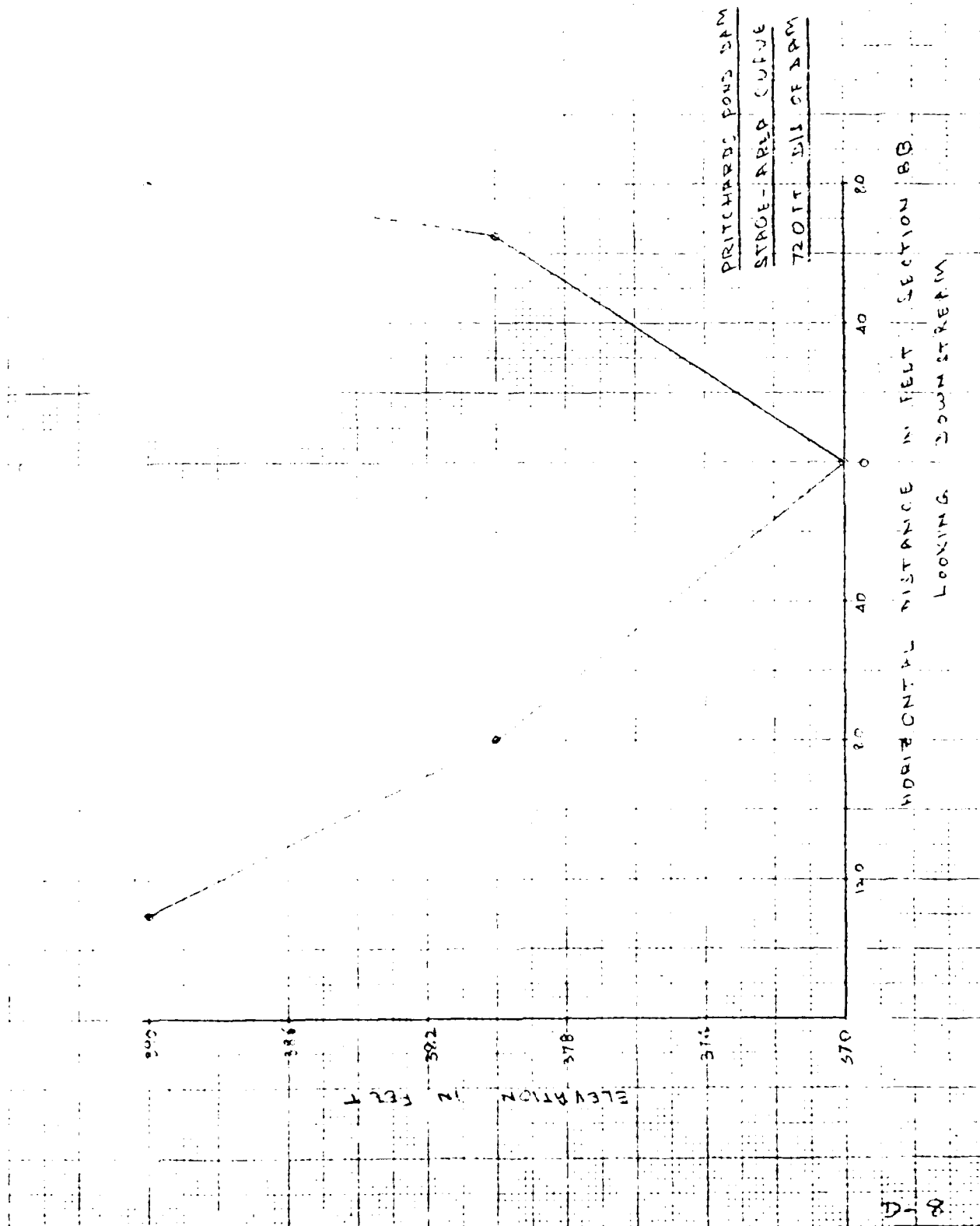
$$= 1185 \left(1 - \frac{3.7}{115}\right) = 1147 \text{ CFS}$$

FOR THIS  $Q_2$ , ELVN = 376.2 AND AREA = 280 SQ. FT.

SHEET 8 OF 16

MA 12/16/20

EB 12/17/80



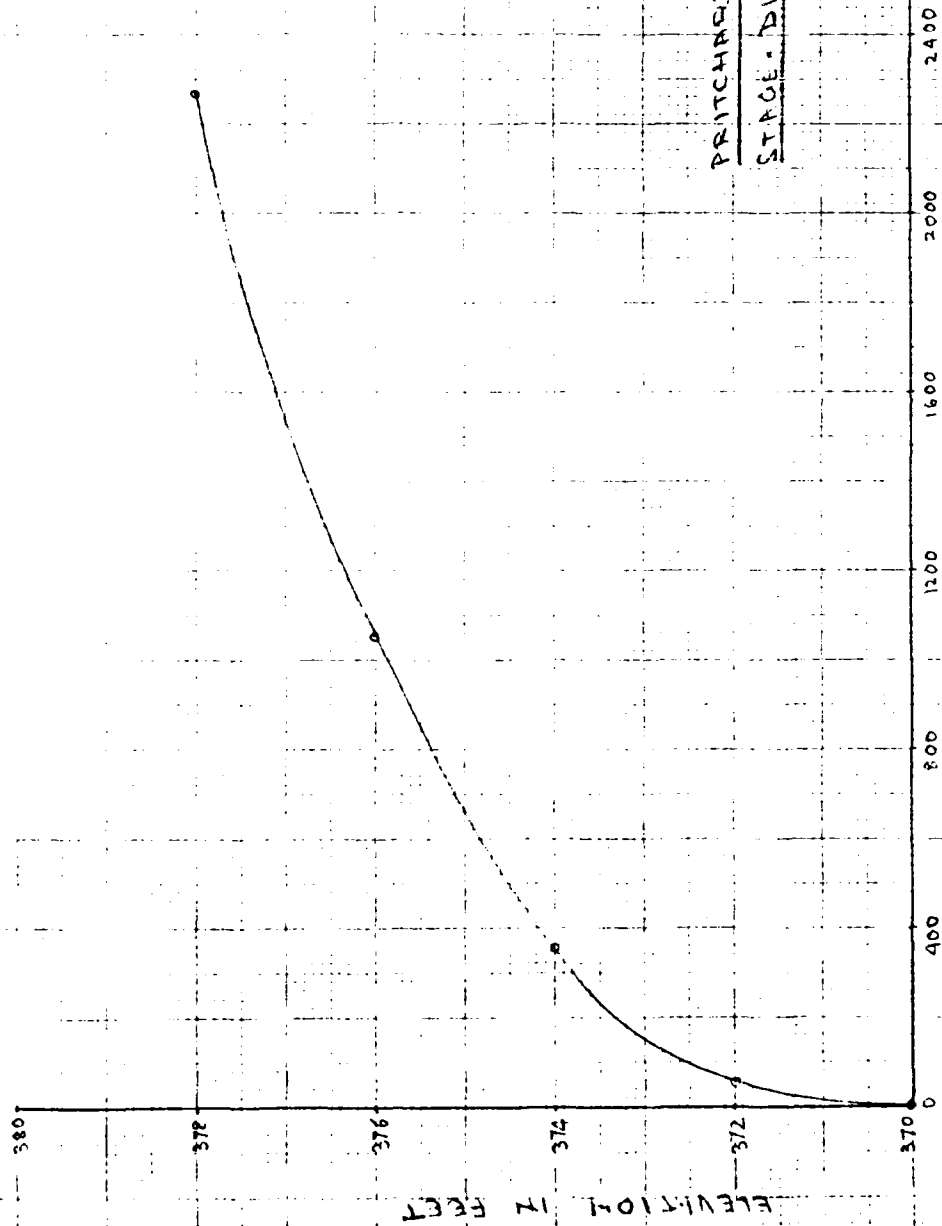
D-8



SHEET 9 OF 16

MA 12/16/80  
EB 12/17/80

PRITCHARDS POND DAM  
STAGE - DISCHARGE CURVE



# DIVERSIFIED TECHNOLOGIES CORP.

CONSULTING ENGINEERS  
NORTH HAVEN, CONN.

PROJECT NON FEDERAL DAM INSPECTION PROJECT NO. 80-13-11 SHEET 10 OF 16  
NEW ENGLAND DIVISION COMPUTED BY MD DATE 12/16/20  
PRITCHARDS POND DAM CHECKED BY CL DATE 12/17/22

$$\text{VOLUME OF REACH } V_2 = \frac{550 \times 280}{43.560} = 3.0 \text{ AC. FT.}$$

$$\text{RECOMPUTING } QP_2 = 1185 \left(1 - \frac{3.7 + 3.5}{2}\right) = 1148 \text{ CFS}$$

$$\text{FLOOD STAGE} = 376.2 \text{ NAVD}$$

$$\text{FLOOD DEPTH} = 6.2 \text{ FT.}$$

$$\text{VELOCITY} = \frac{1148}{280} = 4.1 \text{ FPS}$$

THE TWO HOUSES ADJACENT TO THE SMALL POND  
LOCATED AT SECTION BB ARE HIGHER THAN THE  
ESTIMATED FLOOD STAGE; THEREFORE ARE NOT  
LIKELY TO BE IMPACTED BY DAM FAILURE.

## SECTION CC

THIS SECTION IS SELECTED 1600' DIS FROM SECTION BB  
USING MANNING'S EQUATION

$$Q = \frac{1.486}{n} A R^{2/3} \quad \text{WHERE } n = 0.09 \text{ ASSUMED} \\ = 2.63 A R^{2/3} \quad \text{AND } R = 0.02 \text{ EST. FROM USGS MAP}$$

ELVN	A SQ. FT	P	R	R <sup>2/3</sup>	Q CFS
334	0	—	—	—	—
336	60	60	1	1	160
338	240	120	2	1.6	1010
340	525	175	3	2.08	2870

FOR PEAK FAILURE OUTFLOW  $QP_1 = 1148 \text{ CFS}$

ELVN FROM STAGE-DISCHARGE CURVE = 338.2

AND STAGE AREA CURVE GIVES AREA = 262 SQ. FT.

FOR A REACH LENGTH OF 2000 FT,

$$V_1 = \frac{2000 \times 262}{43.560} \approx 12 \text{ AC. FT.}$$

PROJECT NON FEDERAL DAM INSPECTION PROJECT NO. 80-13-11 SHEET 11 OF 11  
NEW ENGLAND DIVISION COMPUTED BY MP DATE 12/13/80  
PRITCHARDS POND DAM CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_

$$\begin{aligned} \text{TRIAL } QP_2 &= QP_1 \left(1 - \frac{V_1}{5}\right) \\ &= 1148 \left(1 - \frac{12}{11.5}\right) = 1028 \text{ CFS} \end{aligned}$$

FOR THIS  $QP_2$  ELVN FROM DISCHARGE CURVE

$$= 338.05 \text{ AND AREA} = 24359 \text{ FT.}$$

$$\text{VOLUME OF REACH } V_2 = \frac{2000 \times 243}{43.560} \approx 11.2 \text{ AC.FT.}$$

$$\text{RECOMPUTING } QP_2 = 1148 \left(1 - \frac{12 \times 11.2}{2 \times 11.5}\right) = \underline{1032 \text{ CFS}}$$

<u>FLOOD STAGE</u>	<u>= 338.1 NGVD</u>
<u>FLOOD DEPTH</u>	<u>= 4.1 FT.</u>
<u>VELOCITY</u>	<u>= <math>\frac{1032}{243} = 4.25 \text{ FPS}</math></u>

NO DAMAGE IS EXPECTED TO OCCUR  
IN THIS REACH.

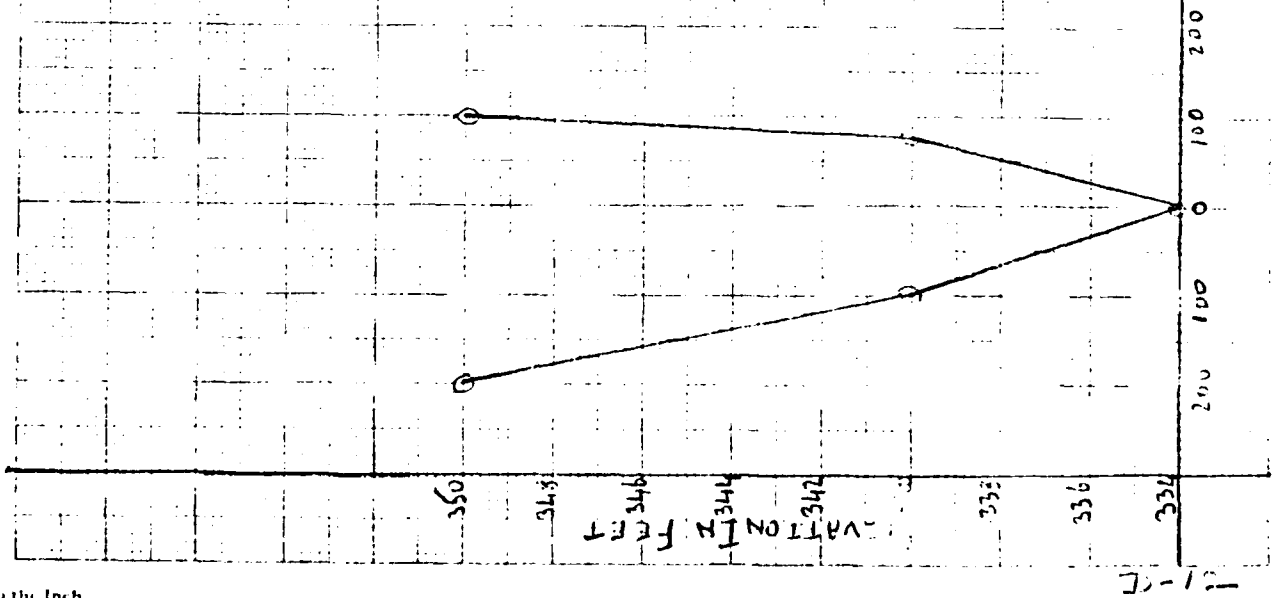
SHEET 12 OF 16

MA 12/16/80  
ED 12/17/80

PRITCHARD'S POND DAM  
STAGE - AREA CURVE  
2320 FT DIS OF DAM

SECTION CC

HORIZONTAL DISTANCE IN FEET  
LOOKING DOWNSTREAM

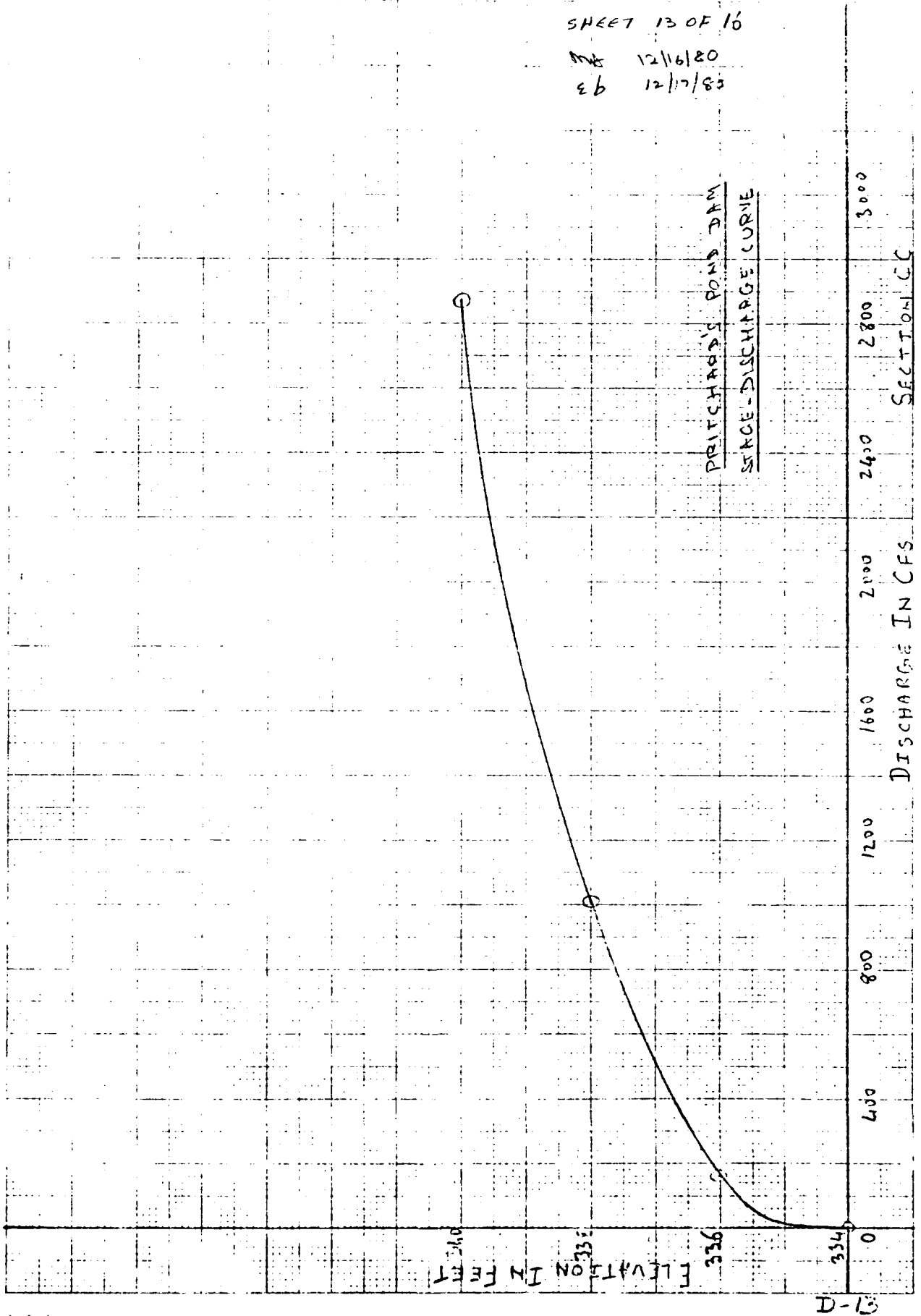


SHEET 13 OF 16

12/16/80

12/17/83

PRITCHARD'S POND DAM  
STAGE-DISCHARGE CURVE



D-13

# DIVERSIFIED TECHNOLOGIES CORP.

CONSULTING ENGINEERS  
NORTH HAVEN, CONN.

PROJECT NON FEDERAL DAM INSPECTION PROJECT NO. 80-13-11 SHEET 14 OF 16  
NEW ENGLAND DIVISION COMPUTED BY MA DATE 12/16/82  
PRITCHARDS POND DAM CHECKED BY CB DATE 12/17/82

SECTION DD IS SELECTED 950' DIS OF CC  
ADJACENT TO MERRITT STREET.  
USING MANNING'S EQUATION

$$Q = \frac{1.486}{n} \times A \times R^{2/3} \times S^{1/2} \quad \text{WHERE } n = 0.06 \text{ ASSUMED}$$

$$S = 0.014 \text{ EST. FROM USGS MAP}$$

$$= 2.93 \times A \times R^{2/3}$$

ELVN	A SQ. FT	P	R	R <sup>2/3</sup>	Q CFS
323	0	—	—	—	—
324	26	52	0.5	0.63	48
325	100	100	1	1	293
326	231	154	1.5	1.31	886
327	400	200	2	1.6	1,875

FOR PEAK FAILURE OUTFLOW  $Q_{P1} = 1032 \text{ CFS}$ , THE STAGE  
DISCHARGE CURVE GIVES ELVN = 326.18 AND AREA  
= 270 SQ. FT.

FOR A REACH LENGTH OF 200 FT,  
VOLUME OF REACH  $V_1 = \frac{200 \times 270}{43.560} \cong 1.2 \text{ AC. FT.}$

$$\text{TRIAL } Q_{P2} = Q_{P1} \left(1 - \frac{V_1}{S}\right)$$

$$= 1032 \left(1 - \frac{1.2}{115}\right) = 1021 \text{ CFS}$$

FOR THIS  $Q_{P2}$  ELVN = 326.16 AND AREA = 264 SQ. FT.  
VOLUME OF REACH  $V_2 = \frac{200 \times 264}{43.560} \cong 1.2 \text{ AC. FT.}$

∴ PEAK OUTFLOW  $Q_{P2} = 1021 \text{ CFS.}$

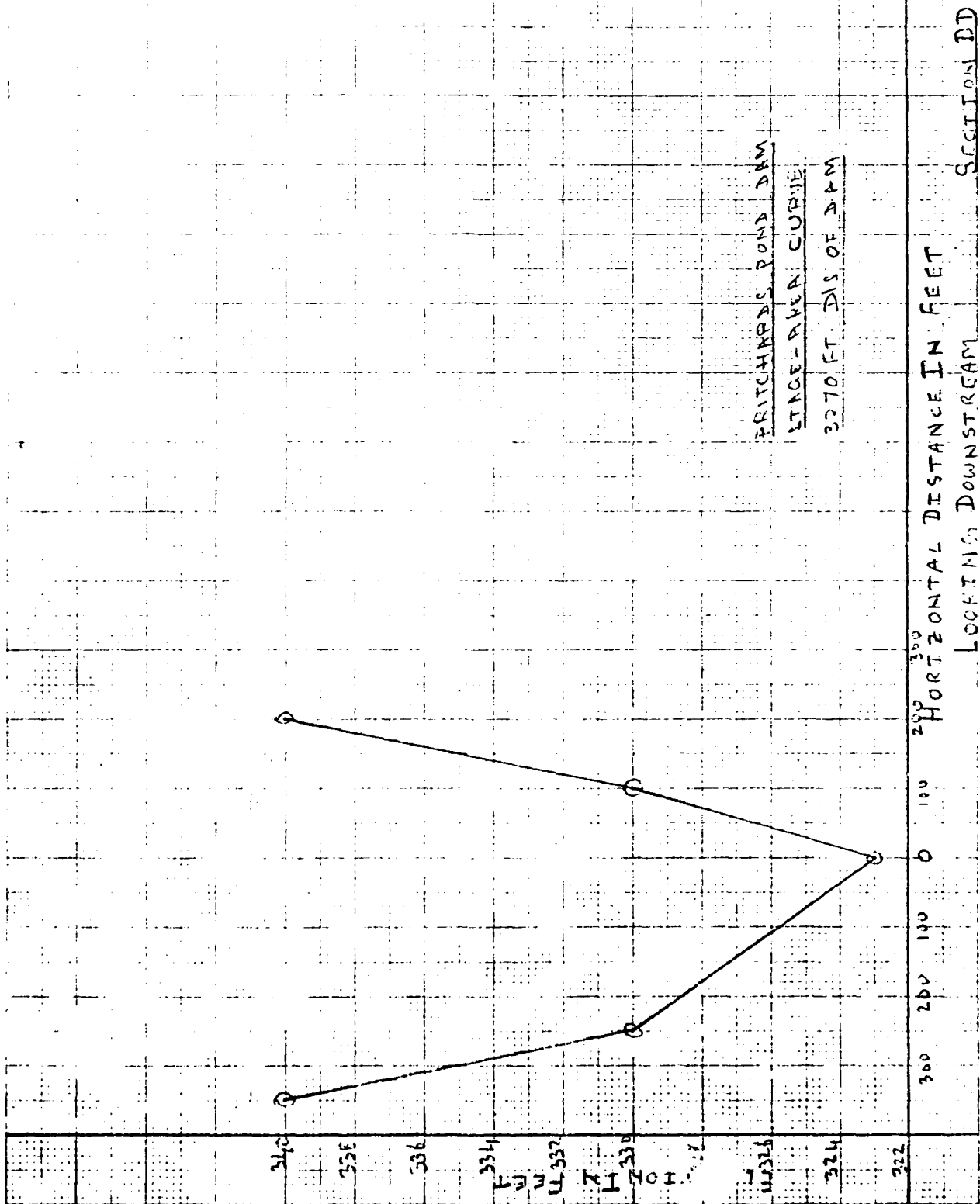
FLOOD STAGE  $\cong 326.2 \text{ NGVD}$   
FLOOD DEPTH  $\cong 3.2 \text{ FT.}$   
VELOCITY  $= \frac{1021}{264} \cong 3.9 \text{ FPS}$

THE HOUSES IN THIS VICINITY ARE 5' FT ABOVE  
THE BED OF THE BROOK.

SHEET 15 OF 16

MA 12/16/80

EB 12/17/80



PITCHER'S POND DAM

STAGE-AVER COURSE

3270 FT. DIS OF DAM

HORIZONTAL DISTANCE IN FEET

LOOKING DOWNSTREAM SECTION DD

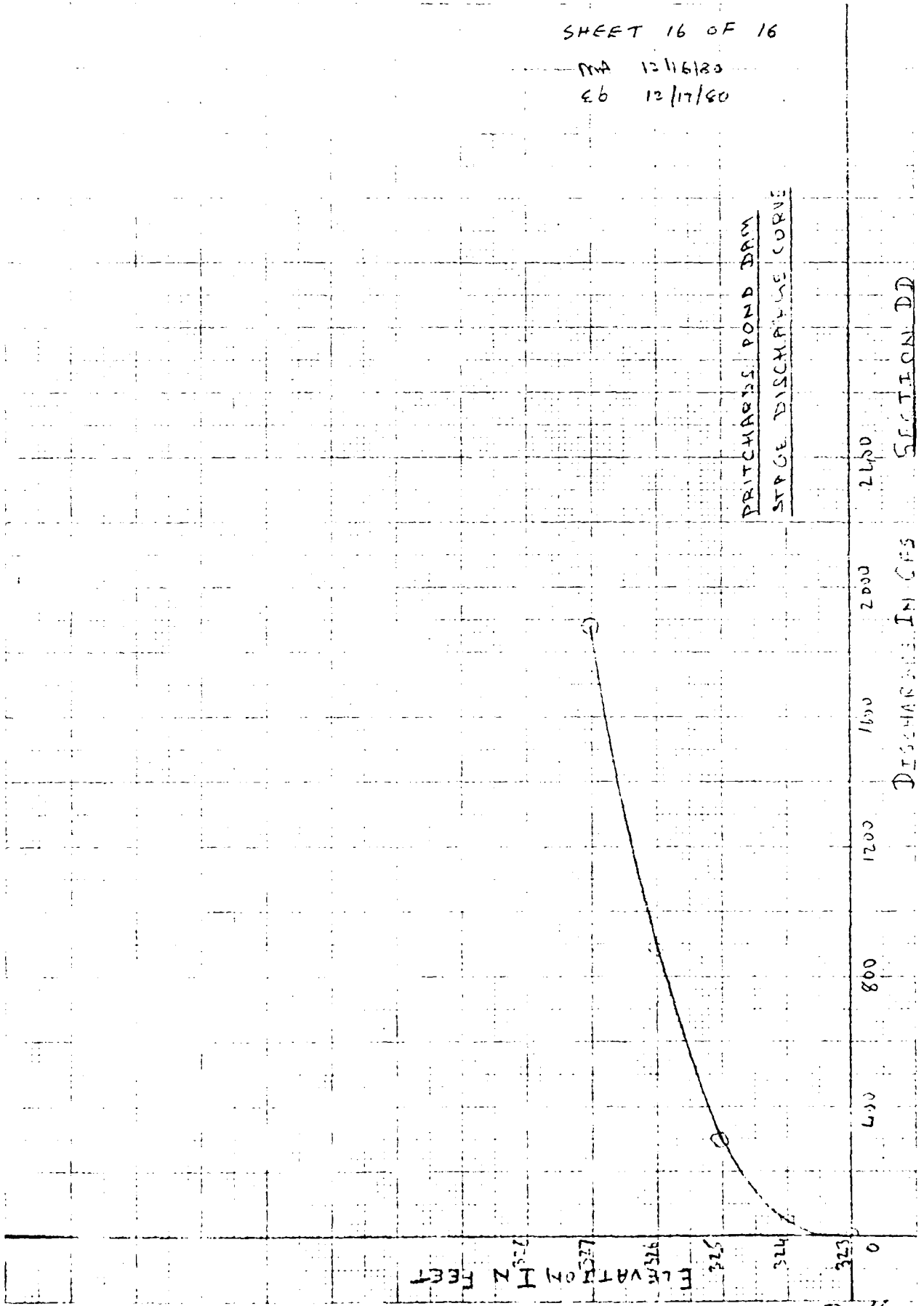
SHEET 16 OF 16

MA 12/16/80

EB 12/17/80

PITCHER'S POND DAM  
STAGE DISCHARGE CURVE

DISCHARGE IN CFS SECTION DD



D-76



APPENDIX E  
VISUAL CHECK LIST WITH COMMENTS

VISUAL INSPECTION CHECK LIST  
PARTY ORGANIZATION

PROJECT PRITCHARD's POND DAM

DATE December 3, 1980

TIME 8-10:30 a.m.

WEATHER Overcast, 33°F.

W.S. ELEV. \_\_\_\_\_ U.S. \_\_\_\_\_ D.W.S. \_\_\_\_\_

PARTY:

- |                                     |           |
|-------------------------------------|-----------|
| 1. <u>Walt Gancarz - Genovese</u>   | 6. _____  |
| 2. <u>Mark Ballou - Genovese</u>    | 7. _____  |
| 3. <u>Murali Atluru - DTC</u>       | 8. _____  |
| 4. <u>Richard F. Murdock - GEI</u>  | 9. _____  |
| 5. <u>Richard W. Turnbull - GEI</u> | 10. _____ |

PROJECT FEATURE	INSPECTED BY	REMARKS
1. <u>Embankment</u>	<u>All</u>	
2. <u>Outlet works</u>	<u>All</u>	
3. <u>Spillway</u>	<u>All</u>	
4. _____		
5. _____		
6. _____		
7. _____		
8. _____		
9. _____		
10. _____		

# PERIODIC INSPECTION CHECK LIST

PROJECT PRITCHARD'S POND DAM

DATE December 3, 1980

PROJECT FEATURE Dam Embankment

NAME \_\_\_\_\_

DISCIPLINE Geotechnical, Civil/Str.

NAME WG, RFM, RWT

AREA EVALUATED	CONDITIONS
<u>DAM EMBANKMENT</u>	Earth embankment with downstream stone masonry wall.
Crest Elevation	386.7
Current Pool Elevation	382.5
Maximum Impoundment to Date	
Surface Cracks	None observed.
Pavement Condition	Asphalt pavement moderately cracked.
Movement or Settlement of Crest	Minor undulations of crest surface.
Lateral Movement	None observed.
Vertical Alignment	Good.
Horizontal Alignment	Good.
Condition at Abutment and at Concrete Structures	Two trees near right abutment (12 and 36 in. diameter).
Indications of Movement of Structural Items on Slopes	None observed.
Trespassing on Slopes	Footpath and scattered trash on slope.
Sloughing or Erosion of Slopes or Abutments	Minor sloughs and erosion gullies upstream slope of embankment.
Rock Slope Protection - Riprap Failures	No slope protection.
Unusual Movement or Cracking at or near Toes	None observed.
Unusual Embankment or Downstream Seepage	Wet area and minor seepage observed left floodplain about 50 ft. downstream of embankment. Minor seepage masonry wall, adjacent to outlet.
Piping or Boils	None observed.
Foundation Drainage Features	None observed.
Toe Drains	None observed.
Instrumentation System	None.
Vegetation	Scattered trees, light brush and on crest and upstream slope.

# PERIODIC INSPECTION CHECK LIST

PROJECT PRITCHARD'S POND DAM DATE December 3, 1980

PROJECT FEATURE Dike Embankment NAME \_\_\_\_\_

DISCIPLINE \_\_\_\_\_ NAME \_\_\_\_\_

AREA EVALUATED	CONDITION
<u>DIKE EMBANKMENT</u>	None.
Crest Elevation	
Current Pool Elevation	
Maximum Impoundment to Date	
Surface Cracks	
Pavement Condition	
Movement or Settlement of Crest	
Lateral Movement	
Vertical Alignment	
Horizontal Alignment	
Condition at Abutment and at Concrete Structures	
Indications of Movement of Structural Items on Slopes	
Trespassing on Slopes	
Sloughing or Erosion of Slopes or Abutments	
Rock Slope Protection - Riprap Failures	
Unusual Movement or Cracking at or near Toes	
Unusual Embankment or Downstream Seepage	
Piping or Boils	
Foundation Drainage Features	
Toe Drains	
Instrumentation System	
Vegetation	

# PERIODIC INSPECTION CHECK LIST

PROJECT PRITCHARD'S POND DAM

DATE December 3, 1980

PROJECT FEATURE Outlet Works- Intake

NAME \_\_\_\_\_

DISCIPLINE Civil/Str.

NAME WG

AREA EVALUATED	CONDITION
<p><u>OUTLET WORKS - INTAKE CHANNEL AND INTAKE STRUCTURE</u></p> <p>a. Approach Channel</p> <p>Slope Conditions</p> <p>Bottom Conditions</p> <p>Rock Slides or Falls</p> <p>Log Boom</p> <p>Debris</p> <p>Condition of Concrete Lining</p> <p>Drains or Weep Holes</p> <p>b. Intake Structure</p> <p>Condition of Concrete</p> <p>Stop Logs and Slots</p>	<p>Not visible (under water).</p> <p>Poor.</p> <p>Clogged with debris - no longer working</p>

# PERIODIC INSPECTION CHECK LIST

PROJECT PRITCHARD'S POND DAM

DATE December 3, 1980

PROJECT FEATURE Outlet Works - Control Tower NAME \_\_\_\_\_

DISCIPLINE \_\_\_\_\_

NAME \_\_\_\_\_

AREA EVALUATED	CONDITION
<u>OUTLET WORKS - CONTROL TOWER</u>	None observed.
a. Concrete and Structural	
General Condition	
Condition of Joints	
Spalling	
Visible Reinforcing	
Rusting or Staining of Concrete	
Any Seepage or Efflorescence	
Joint Alignment	
Unusual Seepage or Leaks in Gate Chamber	
Cracks	
Rusting or Corrosion of Steel	
b. Mechanical and Electrical	
Air Vents	
Float Wells	
Crane Hoist	
Elevator	
Hydraulic System	
Service Gates	
Emergency Gates	
Lightning Protection System	
Emergency Power System	
Wiring and Lighting System	

# PERIODIC INSPECTION CHECK LIST

PROJECT PRITCHARD'S POND DAM

DATE December 3, 1980

PROJECT FEATURE Outlet Works - Conduit

NAME \_\_\_\_\_

DISCIPLINE Civil/Str.

NAME WG

AREA EVALUATED	CONDITION
<p><u>OUTLET WORKS - TRANSITION AND CONDUIT</u></p> <p>General Condition of Concrete</p> <p>Rust or Staining on Concrete</p> <p>Spalling</p> <p>Erosion or Cavitation</p> <p>Cracking</p> <p>Alignment of Monoliths</p> <p>Alignment of Joints</p> <p>Numbering of Monoliths</p>	<p>6" Cast Iron Pipe protruding from d/s face of dam.</p> <p>E-6</p>

# PERIODIC INSPECTION CHECK LIST

PROJECT PRITCHARD'S POND DAM

DATE December 3, 1980

PROJECT FEATURE Outlet Works - Str./Channel

NAME \_\_\_\_\_

DISCIPLINE Geotechnical

NAME RFM, RWT

AREA EVALUATED	CONDITION
<p><u>OUTLET WORKS - OUTLET STRUCTURE AND</u>  <u>OUTLET CHANNEL</u></p>	
<p>General Condition of Concrete</p>	
<p>Rust or Staining</p>	
<p>Spalling</p>	
<p>Erosion or Cavitation</p>	
<p>Visible Reinforcing</p>	
<p>Any Seepage or Efflorescence</p>	
<p>Condition at Joints</p>	
<p>GEI Drain holes</p>	<p>None observed.</p>
<p>GEI Channel</p>	<p>Banks lined with stone wall.</p>
<p>GEI Loose Rock or Trees Overhanging Channel</p>	<p>Parts of stone wall bank liner are loose</p>
<p>GEI Condition of Discharge Channel</p>	<p>Partially blocked with cluster of 5 tree</p>
	<p>joined at base (6"-8" diameter), and</p>
	<p>by several boulders which have fallen</p>
	<p>off left bank wall into discharge channel</p>
	<p>E-7</p>



# PERIODIC INSPECTION CHECK LIST

PROJECT PRITCHARD'S POND DAM

DATE December 3, 1980

PROJECT FEATURE Outlet Works- Weir

NAME \_\_\_\_\_

DISCIPLINE Civil/Str, Hydraulic

NAME WG, MA

AREA EVALUATED	CONDITION
<u>OUTLET WORKS - SPILLWAY WEIR, APPROACH AND DISCHARGE CHANNELS</u>	
a. Approach Channel	Not Visible (under water)
General Condition	
Loose Rock Overhanging Channel	
Trees Overhanging Channel	
Floor of Approach Channel	
b. Weir and Training Walls	
General Condition of Concrete	Good. trash rack is clogged with debris
Rust or Staining	
Spalling	
Any Visible Reinforcing	
Any Seepage or Efflorescence	
Drain Holes	
c. Discharge Channel	
General Condition	Good
Loose Rock Overhanging Channel	Parts of stone wall are loose
Trees Overhanging Channel	Yes - 5 trees immediately d/s of outlet.
Floor of Channel	Clear (except for trees)
Other Obstructions	No

# PERIODIC INSPECTION CHECK LIST

PROJECT PRITCHARD'S POND DAM

DATE December 3, 1980

PROJECT FEATURE Outlet works - Service Bridge

NAME \_\_\_\_\_

DISCIPLINE \_\_\_\_\_

NAME \_\_\_\_\_

## AREA EVALUATED

## CONDITION

### OUTLET WORKS - SERVICE BRIDGE

None observed.

#### a. Super Structure

Bearings

Anchor Bolts

Bridge Seat

Longitudinal Members

Under Side of Deck

Secondary Bracing

Deck

Drainage System

Railings

Expansion Joints

Paint

#### 1. Abutment & Piers

General Condition of Concrete

Alignment of Abutment

Approach to Bridge

Condition of Seat & Backwall

DATE  
FILMED  
8

GEI	Foundation Drainage features		
GEI	Toe Drains		None observed.
GEI	Instrumentation System	E-2	None.
GEI	Vegetation		Scattered trees on crest and up



